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# RETRACTED: Size effect of mechanical characteristics of sandstone and granite under uniaxial compression

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The accurate evaluation of the mechanical properties of rocks is crucial to the design of large-scale projects, such as the construction of dams and bridges, the excavation of tunnels and petroleum engineering research. In this study, experiments pertaining to the size effect of sandstone and granite were performed on small square samples of 10–25 mm. It was found that the elastic modulus of sandstone and granite decreases significantly with increasing size. The uniaxial compressive strength, however, follows a different trend. When the size of the sample increases from 10 to 25 mm, the uniaxial compressive strength of sandstone increases by approximately 12%. However, the strength of granite decreases by 35% as the size increases. The discrepancies in the failure of sandstone and granite are apparent, and the failure patterns of the same rock do not vary with size. Sandstone mostly suffers from split damage, whereas granite mainly suffers from shear failure. Contemporary studies have found that the mechanical properties of rocks are closely related to the sizes of the samples. However, these studies have primarily focused on the size effect of rocks, whose samples sizes were greater than 30 mm in diameter. The understanding constructs the relationship between the strength transformation of different sizes of rocks, which is of great significance for practical engineering applications.

## KEYWORDS

granite, sandstone, size effect, uniaxial compression, square samples

## 1 Introduction

Existing laboratory test standards prescribed by the International Society of Rock Mechanics recommends the use of cylindrical cores having a height-to-diameter ratio of 2.0:2.5 for uniaxial compression testing to obtain the mechanical parameters and failure characteristics of rocks. However, researchers (Mogi, 1962; Koifman, 1963; Hoek and Bieniawski, 1965; Lundborg, 1967; Zdeněk, 1984; Baochen et al., 1998; Mingqing and Youfeng, 2000; Pells, 2004; Celik, 2017; Lianshan et al., 2017; Gang et al., 2018) have also found that the size of a tested sample has a significant influence on the mechanical properties of rock.

Rock samples of different sizes often show tremendous variations in their mechanical properties; this is known as the *size effect* (Zdeněk, 1984; Baochen et al., 1998; Masoumi et al., 2016; Masoumi et al., 2017). Studies on the topic emerged in the 1950s, and many scholars

(Hoek and Bieniawski, 1965; Abou-Sayed, 1976; Lundborg, 1967; Bieniawski, 1968; Bieniawski, 1973) have found that the uniaxial compressive strength of different rocks increase with their reduced size. According to the uniaxial test results performed on various rocks, Hoek (Judd, 1983; Darlington et al., 2011) created the Hoek–Brown model, in which the strength decreases exponentially with the increase in size. Through the uniaxial test results of cylindrical granite, Lundborg, (1967) similarly found that the strength of the rock increases with reduction in its size. This trend was confirmed by Weibull, (1951) theory. Liu (Baochen et al., 1998) believed that the strength of dissimilar rocks varied significantly with their decreased size. A few researchers also found that the strength of the rock samples did not continue to vary with the increase in size, and when a specific critical size was reached, the rock strength ceased to change. Hoek (Darlington et al., 2011) assumed that when a rock block size was much smaller than its mass, the strength of the mass would reach a stable minimum. Merwe, (2003) found that the strength of coal increased exponentially with its decrease in size, but the size effect was not significant in samples having diameters over 2,000 mm. Bieniawski, (1968) discovered that when the size of a coal block was smaller than 65 mm or larger than 1,500 mm, the strength remained unchanged. Pratt et al. (1972) detected that when the dimension of quartz diorite exceeded 900 mm, the rock strength was nearly constant. In 1998, Hawkins, (1999) conducted a series of uniaxial compression tests on sedimentary rocks, concluding that the strength beyond the diametric range of 40–60 mm was significantly lower than that within the prescribed range. While studying the size effect of rocks, various scholars (Mogi, 1962; Hoek and Bieniawski, 1965; Bieniawski, 1968; Madan and Peter, 1974; Celik, 2017; Abou-Sayed, 1976) experimented with samples of different shapes, mainly including cylindrical, prismatic, and cubic. Among the factors examined, the size effect of the cylindrical specimens was the most widely studied, and the sizes ranged from 12.5 to 1,650 mm. The size ranges of the cubic and prismatic samples were 10–1,500 mm and 40–2,700 mm, respectively. The study of the above scholars opens the door of rock size effect and provides ideas for the subsequent research work in this paper.

Previous studies on the size effect of rocks mainly examined igneous, sedimentary, and metamorphic types, focusing on cylindrical samples having diameters larger than 30 mm. However, there have been few studies that focused on the size effect of rock samples having sizes less than 30 mm. In this study, granite (an igneous rock) and sandstone (a sedimentary rock) are selected to study the size effect of square samples ranging from 10- to 25 mm. By analyzing the stress–strain curve, elastic modulus, uniaxial compressive strength, and failure mode of rock specimens, the size effect on the mechanical characteristics of these rocks is obtained.

## 2 Experimental method

### 2.1 Rock material

The rock samples used in this paper were taken from a natural outcrop of sandstone in the Longchang of the Sichuan province (105.22°E, 29.30°N) and one of granite in Zhangzhou of the Fujian

Province (117.86°E, 24.83°N). A series of physical parameters were tested on the two types, and the density, porosity, permeability, and acoustic velocity parameters were measured, as shown in Table 1.

To avoid the influence of rock anisotropy on the mechanical parameters of the samples, the tested sandstone and granite were taken from a whole rock in the same direction and processed into cubes. The flatness error for each surface was also controlled and maintained within 0.02 mm. Cubic samples of sandstone and granite were prepared in sizes of 10, 15, 20, and 25 mm, with five samples of each, numbering 40 specimens in total. Processed sandstone and granite samples shown in Figure 1. (B, C, D, and E) represent sample sizes from smallest to the largest.

### 2.2 Test method

In this study, the GCTS RTR-1000 static triaxial servo testing system was used as triaxial mechanical testing equipment, which obtains the axial deformation of the rock by means of a strain gauge. The experiment was conducted at room temperature and a fixed loading rate to guarantee that the displacement loading rate for all samples was controlled at 0.12 mm/min. To avoid the impact of the uneven end of the instrument on the test sample, a spherical seat was fixed between the upper portion of the rock cube and the loading plate to evenly distribute the loading on the surface of the sample.

## 3 Result

### 3.1 Stress–strain curve

Figures 2, 3 show the stress–strain curves of sandstone and granite samples of different sizes under uniaxial compression repeated five times. Because the two rock samples taken were relatively homogeneous, the overall dispersion of the stress–strain curves obtained from rock samples of each size was small. Similar to other rocks, the compression process for sandstone and granite also underwent pore (crack) compaction, elastic deformation, and other stages. As the size increased, the stress–strain curves for both rocks became smoother, with the fluctuation before the peak stress gradually decreasing and a reduction in the downward trend of the curve after the peak. The characteristics of granite were more significant, with the gap of the elastic deformation stage gradually narrowing as the size increased.

### 3.2 Elastic modulus

Figure 4 shows the elastic modulus of the different sizes of sandstone and granite samples. In this paper, it was found that the size had a significant influence on the elastic modulus. From the perspective of data dispersion, the dispersion of small-size rock samples was the largest, and, with the increase of the size, the dispersion degree of data decreased significantly. The elastic modulus of sandstone decreased by approximately 49% from 49.3 GPa for a 10-mm sample to 24 GPa for a 25-mm specimen. The elastic modulus of granite decreased by approximately 52% from 143.3 GPa for a 10-mm specimen to 68.3 GPa for a 25-mm

TABLE 1 Physical parameters of sandstone and granite.

Rock	Density (g/cm <sup>3</sup> )	Porosity (%)	Permeability (mD)	P-wave velocity (km/s)	S-wave velocity (km/s)
Sandstone	2.22	13.62	0.032	2.74	2.34
Granite	2.78	1.17	0.0001	4.61	3.84

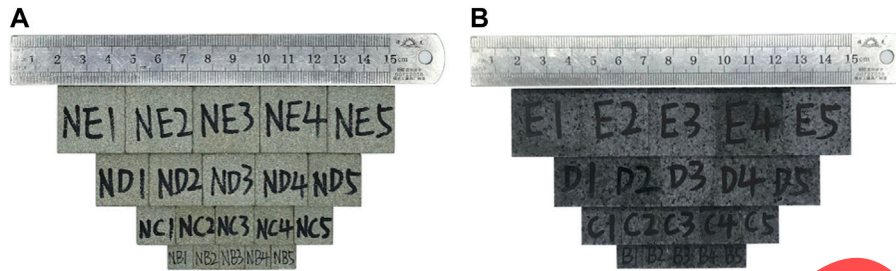


FIGURE 1 Processed rock samples: (A) sandstone samples; (B) granite samples.

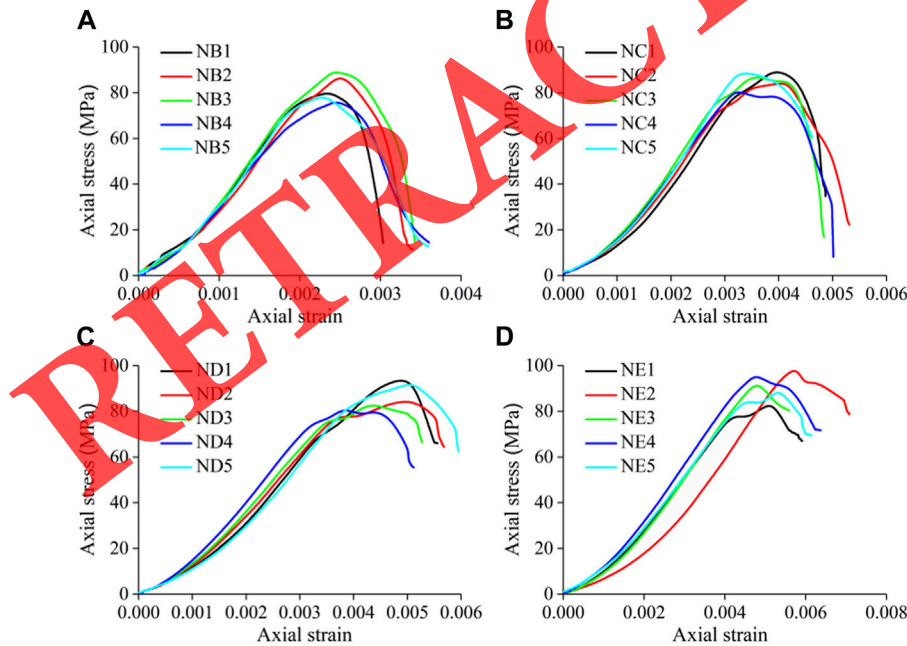


FIGURE 2 Stress-strain curve of sandstone for different sizes: (A) 10-mm samples; (B) 15-mm samples; (C) 20-mm samples; (D) 25-mm samples.

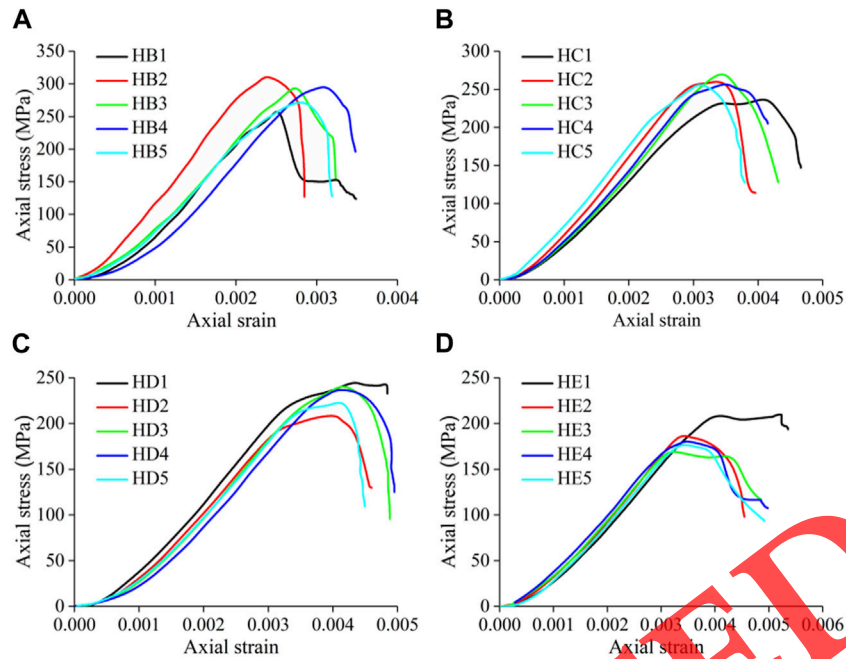
sample. The declining trend of the two rocks was similar, and the rate gradually slowed. With the increase in size, the distribution of defects tended to be uniform, which is essential to reduce the degree of dispersion and the elastic modulus (Jun et al., 2017).

As can be seen from Figure 4, with the increase of size, the average elastic modulus decreased exponentially. Through an

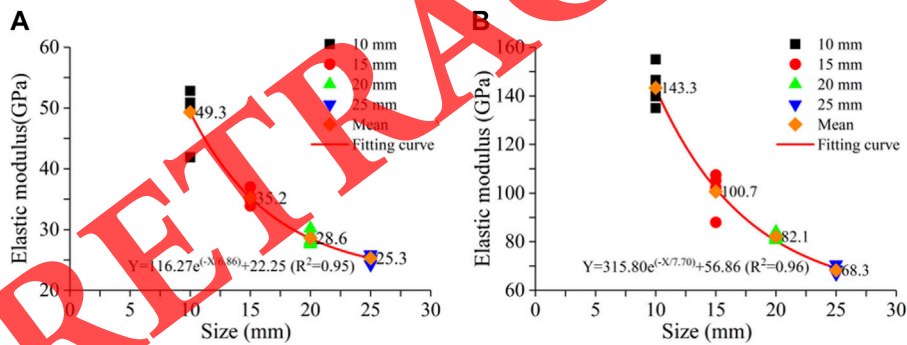
exponential fitting of data, it was found that the curve had good consistency, in which the relationship between elastic modulus and size can be expressed as

$$Y_{sandstone} = 116.27e^{(-X/6.86)} + 22.25 (R^2 = 0.95), \quad (1)$$

$$Y_{granite} = 315.80e^{(-X/7.70)} + 56.86 (R^2 = 0.96). \quad (2)$$



**FIGURE 3** Stress-strain curve of granite for different sizes: (A) 10-mm samples; (B) 15-mm samples; (C) 20-mm samples; (D) 25-mm samples.



**FIGURE 4** Elastic modulus of the rock samples for various sizes: (A) sandstone specimens; (B) granite specimens.

### 3.3 Uniaxial compressive strength

Figure 5 demonstrates the relationship between uniaxial compressive strength and the size of the sandstone and granite specimens. Sample sizes range from 10 mm to 25 mm, and the strength of sandstone and granite varied significantly. For the same-sized samples, the strength of granite was roughly more than twice that of sandstone. With the increase in the size of the sandstone sample, uniaxial compressive strength did not decrease; it rather increased slightly with the increase of sample size, and the average uniaxial compressive strength increased by 12%. However, the granite samples showed a distinct size effect as observed by means of their uniaxial compressive strengths, which showed an

apparent declining trend with increasing size. The data showed that when the size of the sample increased from 10 to 25 mm, the uniaxial compressive strength of granite decreased by 35% from 290 to 187 MPa. In this experiment, the sandstone does not have obvious size effect, but it cannot be ruled out that the sandstone will have obvious size effect in the uniaxial compression experiment of larger size not involved in this study.

The linear fitting of the size and strength relationship of sandstone and granite showed a good consistency, in which the relationship between strength and size can be expressed as

$$Y_{sandstone} = 0.62X + 76.54 (R^2 = 0.946), \tag{3}$$

$$Y_{granite} = -6.68X + 359.69 (R^2 = 0.863). \tag{4}$$

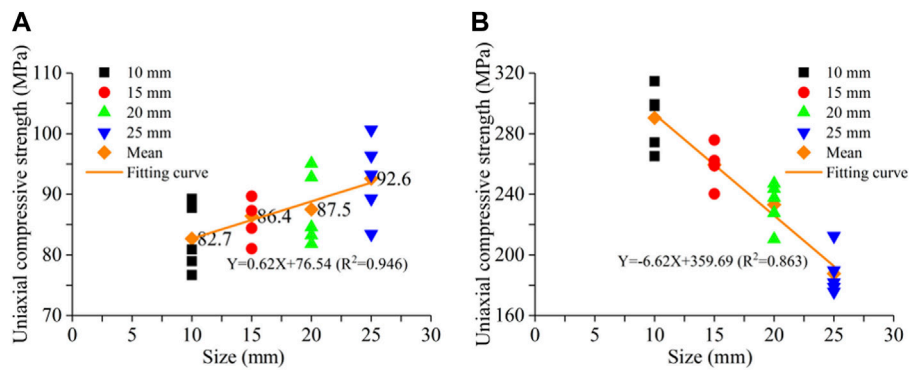


FIGURE 5 Uniaxial compressive strength of the rock samples for various sizes: (A) sandstone specimens; (B) granite specimens.

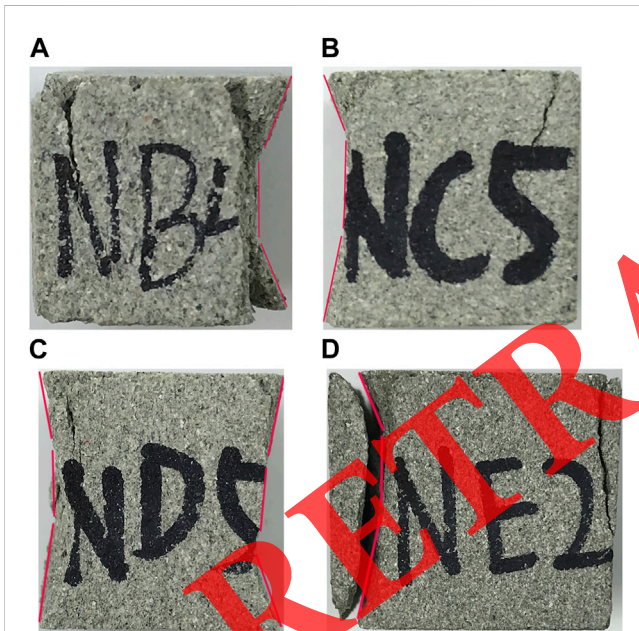


FIGURE 6 Failure pattern of sandstone for different sizes: (A) 10-mm sample; (B) 15-mm sample; (C) 20-mm sample; (D) 25-mm sample.

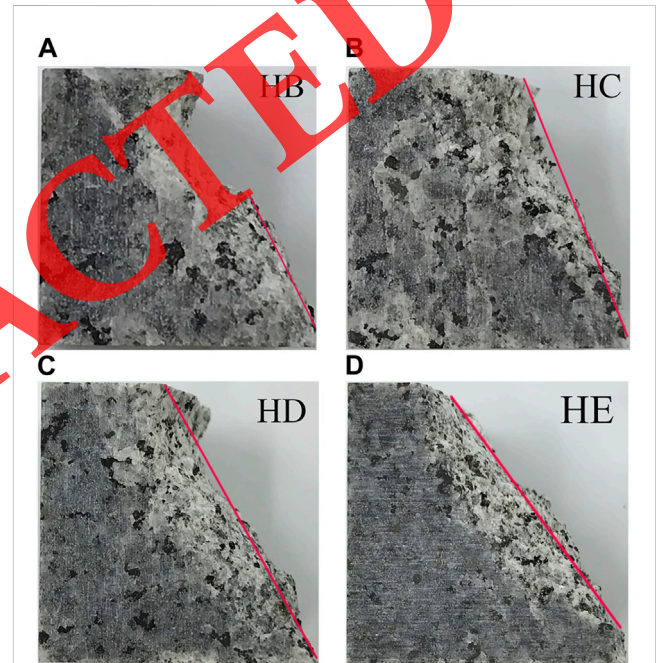


FIGURE 7 Failure pattern of granite for different sizes: (A) 10-mm sample; (B) 15-mm sample; (C) 20-mm sample; (D) 25-mm sample.

### 3.4 Failure pattern

Figure 6 shows the failure modes of sandstone for various sizes. It can be observed that sandstone mainly undergoes split failure and that size has no significant influence on the modes of failure (Basu et al., 2013). In the uniaxial compression, the initial crack first propagates along a plane at a certain angle to the horizontal surface, then the angle gradually increased and finally became nearly vertical. During this process, the microcracks of the upper and lower sections were symmetric. A tensile crack was developed between the two end faces of the sample under pressure. Then, the crack gradually propagated until it extended to the upper and lower end faces, forming a relatively conspicuous crack. At this point, the rock

sample showed an apparent failure, and the stress of the curve showed a downward trend. During the experiment, it was found that a part of the end faces peeled off from the rock. After the experiment, visible cracks were found on the end face of the samples. The maximum crack width was measured to be approximately 1 mm. The end face, however, did not peel off after slight contact, indicating that the crack did not penetrate the entire sample.

Figure 7 shows failed granite samples that underwent shear failure, for which size had no significant influence on the mode of failure, like the sandstone sample. The microcracks propagated along a plane at a certain angle to the horizontal surface with the increase in load. Unlike sandstones, no obvious angle changes were

observed in the crack development of granite. These cracks gradually developed, and the end faces formed tiny flake debris, eventually falling away and breaking the rock along a slope. Most rock samples presented trapezoidal states. However, a few samples of 10 mm presented a single shear failure (Basu et al., 2013). The most significant difference between the two rocks was that there was no apparent crack in the samples after failure, and the samples exhibited complete forms.

## 4 Discussion

It can be seen from the experimental results that, within the size range of 10–25 mm, the elastic modulus of the two rocks decreased exponentially with the increase of size. The research results of different scholars differed among rock types. Jackson (Jackson and Lau, 1991) found that when the diameter of cylindrical Bonnet granite increased from 45 to 300 mm, the Young's modulus decreased by 10%. However, Thuro's (Thuro et al., 2017) research on limestone found that there was no significant size effect on the Young's modulus in samples having a diameter range of 45–80 mm. Yuki's (Yuki, et al., 1995) experimental results on tuff showed that the Young's modulus increased to a certain extent with the increase of sample size in samples having diameters of 30–150 mm. Zhai (Hao et al., 2020) found via uniaxial compression tests on weak rocks, such as cylinder Gambier limestone and artificial rock, that the Young's modulus in 26–285-mm cylindrical limestone and that of 26–139-mm cylindrical artificial rock samples increased gradually with the increase in size until a specific size; then, it decreased. It can be observed that there was a significant difference in the influence rule of size on the elastic modulus. However, the influence mechanism of the rule requires more research.

In this study, we experimented on sandstone and granite under uniaxial compression tests to determine whether size effects exist with obvious differences. We found that the uniaxial compressive strength of sandstone increased by 12% when the size increased from 10 to 25 mm in this paper, which trend is consistent with the results of previous scholars who used the same scale. The researches of sandstone showed that if the size of rock samples increased from 10 to 65 mm, the strength of the rock increases with the increase in size, the increasing range approximate to 34% (Hawkins, 1999) and 70% (Masoumi et al., 2016; Masoumi et al., 2017) respective. However, the compressive strength of granite reduced by about 35% in this paper. Various other scholars who studied similar igneous rock types showed similar results. Bieniawski's (Celik, 2017) test results showed that, when cube samples increased from 10 to 200 mm, the uniaxial compression strength decreased from 295 to 220 MPa which decreasing range close to 25%; Further research is required to explore whether more number of igneous rocks show similar results.

The failure mode results of sandstone and granite rock samples showed significant differences. The sandstone mainly endured axial splitting, whereas the granite suffered mainly shear failure, which is consistent with the test results of (Basu et al., 2013) on cylinder rock samples. The reason for such significant differences is mainly caused by the differences in internal cracks and defects. The shape of the

sample also is an important factor that leads to failure-mode differences (Viso et al., 2008; Gen et al., 2022). For example, the corner of the cube sample was more prone to induce stress concentrations and lead to micro-crack aggregations in the area near the corner. Although the failure mode of the sandstone was mostly related to axial splitting, the cylindrical sandstone mostly axial split in the middle part, whereas the cube rock mostly laterally spalled at the middle section. At present, most of the studies are to reveal the law of the influence of rock size on strength, and further research is needed to investigate the mechanism behind the influence law.

## 5 Conclusion

In this paper, uniaxial compression experiments were conducted on the size effects of square sandstone and granite at 10-, 15-, 20-, and 25-mm sizes. The influences of the size changes of sandstone and granite square samples on the elastic modulus, uniaxial compressive strength, and failure mode were discussed, and the following conclusion were drawn:

1. The size of cubic rock samples has a significant influence on the elastic modulus. The elastic modulus of sandstone and granite decreases exponentially with the increase in size, by 48% and 52%, respectively.
2. The size of cubic rock samples also has a significant influence on the uniaxial compressive strength. The uniaxial compressive strength of granite decreases linearly with the increase in size by up to 35%, showing an apparent size effect. The uniaxial compressive strength of sandstone increases linearly with the increase of its size by 12%.
3. The size change has no significant influence on the failure mode of the rock, and the failure mode of the cube sample is significantly different from that of the cylinder sample, owing to the stress concentration caused by the edges and corners of the cube sample. It was observed that the sandstone is mainly damaged by axial splitting, and the rock sample after the destruction showed a small middle shape with large sides. Granite is mainly damaged by shear, its failure mode is significantly different from that of sandstone, and its failure surface can be approximated as an inclined plane.

## Data availability statement

The original contributions presented in the study are included in the article/Supplementary Material, further inquiries can be directed to the corresponding author.

## Author contributions

LK, JW, and XS contributed to the study conception and design. HW and XZ conducted the numerical simulations and wrote the manuscript. LG and SC conducted the experiments and wrote the first draft of the manuscript. All authors contributed to the article and approved the submitted version.

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## Conflict of interest

Author JW was employed by PetroChina Southwest Oil and Gas Field Company. Author HW was employed by CNPC Engineering Technology R&D Company Limited. The authors declare that this study received funding from Science and Technology Special Project of China Co., Ltd. The funder had

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The remaining authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

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